

Problema 1.

$$\sigma_{rr}(r) = -\frac{\left(\frac{a^2}{r^2} - \frac{a^2}{b^2}\right)}{\left(1 - \frac{a^2}{b^2}\right)} p_i + \frac{\left(\frac{a^2}{b^2} - \frac{a^2}{r^2}\right)}{\left(1 - \frac{a^2}{b^2}\right)} p_o \quad \left. \begin{array}{l} b \rightarrow \infty \\ p_i = p \\ p_o = 0 \end{array} \right\} \Rightarrow \sigma_{rr}(r) = -\frac{(D/2)^2}{r^2} p$$

$$\sigma_{\theta\theta}(r) = \frac{\left(\frac{a^2}{r^2} + \frac{a^2}{b^2}\right)}{\left(1 - \frac{a^2}{b^2}\right)} p_i - \frac{\left(1 + \frac{a^2}{r^2}\right)}{\left(1 - \frac{a^2}{b^2}\right)} p_o \quad \left. \begin{array}{l} b \rightarrow \infty \\ p_i = p \\ p_o = 0 \end{array} \right\} \Rightarrow \sigma_{\theta\theta}(r) = \frac{(D/2)^2}{r^2} p$$

$$u_r(r) = \frac{1-\nu}{E} \frac{\left(p_i \frac{a^2}{b^2} - p_o\right)}{\left(1 - \frac{a^2}{b^2}\right)} r + \frac{1+\nu}{E} \frac{(p_i - p_o)}{\left(1 - \frac{a^2}{b^2}\right)} \frac{a^2}{r} \quad \left. \begin{array}{l} b \rightarrow \infty \\ p_i = p \\ p_o = 0 \end{array} \right\} \Rightarrow \Delta D = 2u_r(D/2) = \frac{1+\nu_f}{E_f} pD$$

Problema 2.

Variação no diâmetro do revestimento (tubo de paredes finas)

$$\Delta D_R = (p_i - p_o) D^2 / 2E_t t$$

Variação no diâmetro do poço (ver Problema 1)

$$\Delta D_f = (1 + \nu_f) p_o D / E_f$$

Compatibilidade de deslocamentos:

$$\Delta D_R = \Delta D_f \Rightarrow p_o = p_i [1 + 2(1 + \nu_f)(tE_t / DE_f)]^{-1}$$

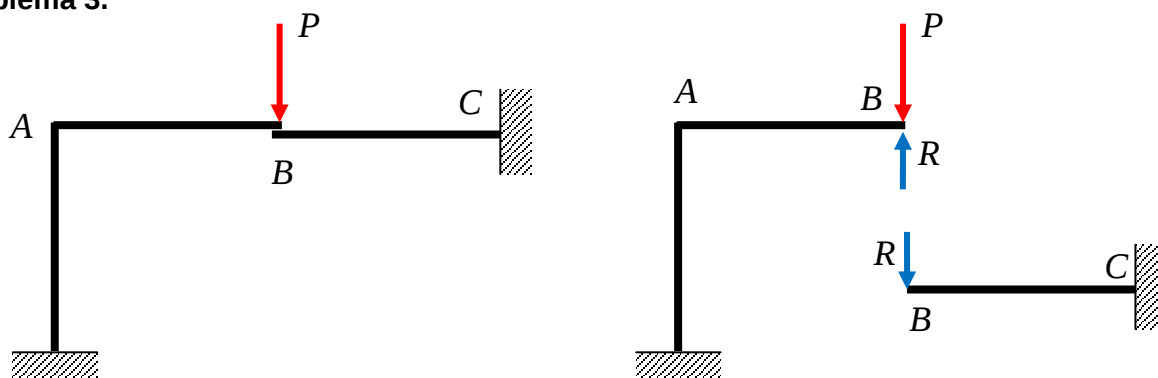
Tensões principais em $r = D/2$

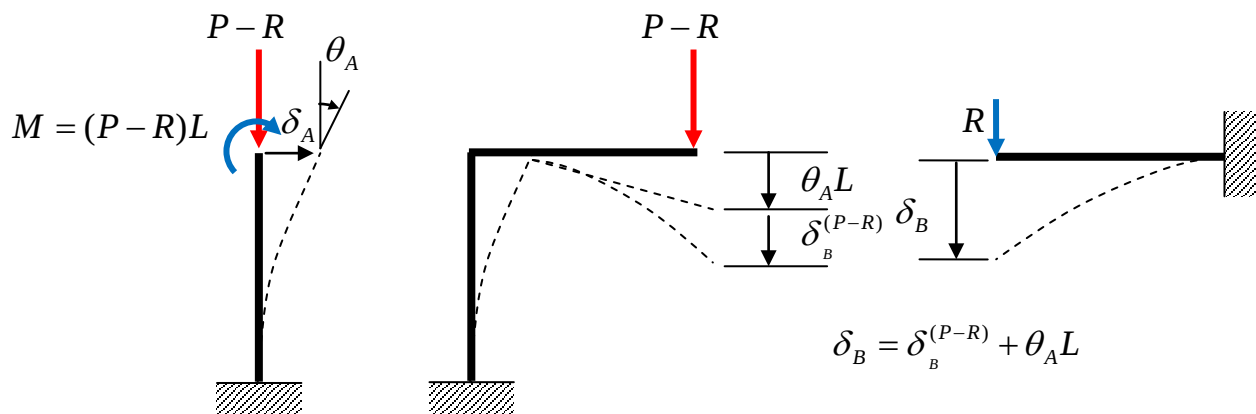
$$\sigma_1 = \sigma_{\theta\theta}(D/2) = p_o, \sigma_2 = 0, \text{ e } \sigma_3 = \sigma_{rr}\left(\frac{D}{2}\right) = -p_o$$

Tensão cisalhante máxima em $r = D/2$

$$\tau_{\max} = (\sigma_1 - \sigma_3)/2 = p_o = p_i [1 + 2(1 + \nu_f)(tE_t / DE_f)]^{-1}$$

Problema 3.





$$\left. \begin{aligned} \delta_B &= \frac{RL^3}{3EI} \\ \delta_B^{(P-R)} &= \frac{(P-R)L^3}{3EI} \\ \theta_A &= \frac{(P-R)L^2}{EI} \end{aligned} \right\} \rightarrow \delta_B = \delta_B^{(P-R)} + \theta_A L \Rightarrow R = \frac{4P}{5} \Rightarrow \boxed{\delta_B = \frac{4PL^3}{15EI}}$$

Problema 4.

(i) $\Delta T < \Delta T_y$ (regime elástico)

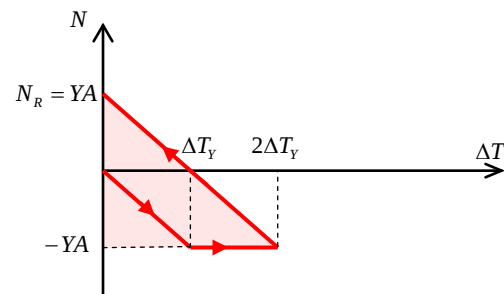
$$\varepsilon = \frac{N}{EA} + \alpha \Delta T = 0 \Rightarrow N = -\alpha EA \Delta T$$

$$\Delta T = \Delta T_y \Rightarrow N = -YA \Rightarrow \boxed{\Delta T_y = Y/\alpha E}$$

(ii) $\Delta T = 0 \rightarrow 2\Delta T_y \rightarrow 0$

$$N_R - (-YA) = \alpha EA (2\Delta T_y) \Rightarrow N_R = \alpha EA (2Y/\alpha E) - YA = YA$$

$$\sigma_R = N_R/A = Y$$



Problema 5.

Assumindo que as deformações plásticas têm início na barra, o colapso ocorre quando tanto a barra quanto a seção do engaste da viga estiverem completamente plastificadas. Neste caso, o momento fletor em A é igual ao momento limite M_L .

Fazendo o balanço de momentos:

$$YA_B L_V - qL_V^2/2 + M_L = 0$$

logo

$$qL = 2M_L/L_V + 2YA_B/L_V$$

